

Mid Fall Term Exam for IPSE of CSE, 2013					20th, Nov., Wednesday			From: 18:15, To:19:45			
Subject		Instructor	Department, Year		Answer	Separate	Reference	Reference tools are not allowed without admission.		<div>1. Nothing</div> <div>2. Free</div> <div>3. Partly allowed</div> <div>• Textbook • Reference book</div> <div>• Calculator</div> <div>• Dictionary</div> <div>• Others [</div>	
Soil Mechanics		H. Akagi	Civil & Env.	2							
Student ID			Name								

Answer all questions (1) ~ (5) on the separate answer sheet. The density of water is $\rho_w=1.00(\text{g/cm}^3)$ and the water unit weight is $\gamma_w=9.81(\text{kN/m}^3)$.

(1) A 1.50 (kg) of dry soil is poured into a Eureka can and displaces 600 (cm^3) of water (see Fig. 1). Find the density of soil particles ρ_s (g/cm^3).

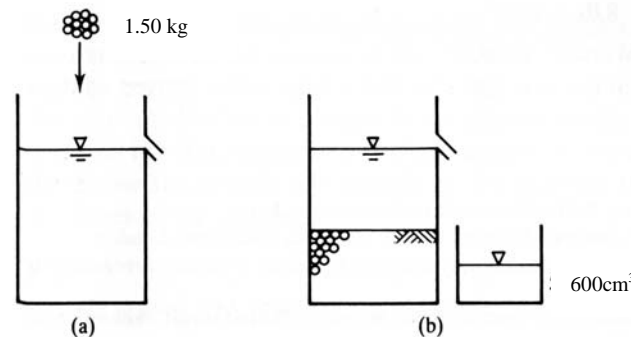


Fig.1

A 160 (g) of the same dry soil is poured uniformly into an empty impermeable metal container and occupied 180 (cm^3) (see Fig. 2). The area of the inside cross section of the container is $A=30.0$ (cm^2). Find the void ratio e of the dry soil in the container.

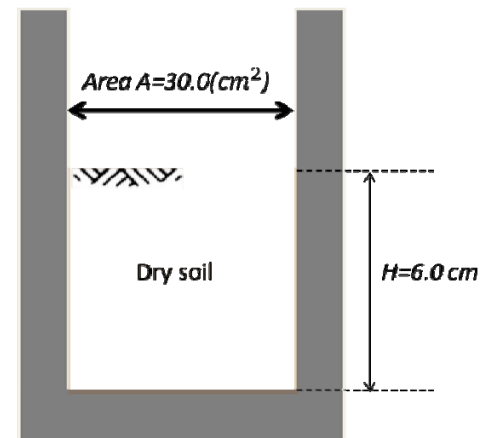


Fig.2

(2) Table 1 shows the results of the density test and water contents test of soil.

Find the density of soil particle $\rho_s(\text{g/cm}^3)$, wet density of soil $\rho_t(\text{g/cm}^3)$, water content $w(\%)$, dry density of soil $\rho_d(\text{g/cm}^3)$, void ratio e and saturation ratio $S_r(\%)$.

Table 1

Types of test	Results	
Density of grains	Volume of the soil particle	11.00 cm^3
Water content	Mass of the wet specimen and container	92.50 g
Wet density	Mass of the specimen and container after drying at 105°C	78.90 g
	Mass of the container	49.30 g
	Volume of the wet specimen	26.20 cm^3

(3) Fig. 2 shows the state of stresses in an element of soil. (Positive shear stress and angles indicate counterclockwise direction.)

1) Calculate the maximum and minimum principal stresses $\sigma_1(\text{kPa})$ and $\sigma_3(\text{kPa})$.

2) Calculate the angle $\alpha(\text{degrees})$ formed by the plane of the maximum principal stress and the horizontal plane.

3) The angle between plane A and horizontal plane within an element of soil is +45 degrees as shown in Fig. 2. Calculate normal and shear stresses $\sigma_A(\text{kPa})$ and $\tau_A(\text{kPa})$ acting on the plane A.

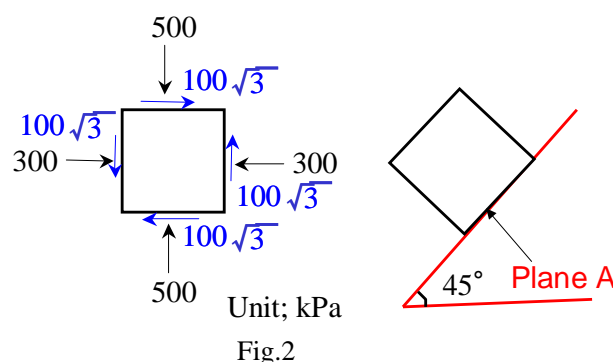


Fig.2

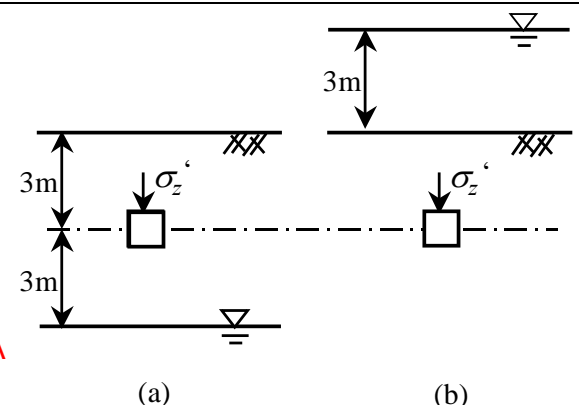


Fig.3

(4) The clay deposit in Fig.3 has unit weight $\gamma_{\text{sat}}=20(\text{kN/m}^3)$ and the soil remains saturated if the pore pressures become negative.

1) When water table is 6 m below ground level as shown in Fig.3(a), find the normal effective stress $\sigma'_{zA}(\text{kN/m}^2)$ at a depth of 3m.

2) When water table is 3 m above ground level as shown in Fig.3(b), find the normal effective stress $\sigma'_{zB}(\text{kN/m}^2)$ at a depth of 3m.

(5) Fig.4 shows the one-dimensional water flow in the sand specimen. Datum line of potential water head z is at the central depth of the sand specimen.

Find the total water head $h_a(\text{m})$, $h_b(\text{m})$ and water flow volume $Q(\text{m}^3/\text{s})$.

End of questions.

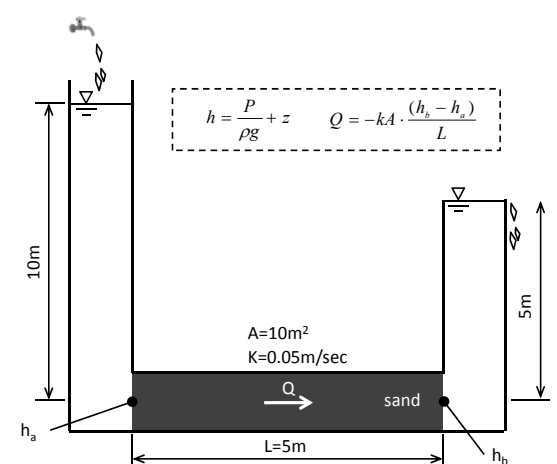


Fig.4

International Program, Department of Civil and Environmental Engineering

Answer sheet for Mid Fall Term Exam, Soil Mechanics, 2013

Student ID _____ Name _____ Mark _____

Question No. (1)	$\rho_s = \frac{1500(\text{g})}{600(\text{cm}^3)} = 2.5(\text{g/cm}^3),$ $v_s = \frac{160(\text{g})}{2.5(\text{g/cm}^3)} = 64(\text{cm}^3), \quad v_v = 180 - 64 = 116(\text{cm}^3), \quad e = \frac{116}{64} \div 1.81$ <p style="text-align: right;">$10 \times 2 = 20$</p>
Question No. (2)	$\rho_s = \frac{78.90 - 49.30}{11.00} \div 2.69(\text{g/cm}^3), \quad \rho_t = \frac{92.50 - 49.30}{26.20} = 1.65(\text{g/cm}^3),$ $w = \frac{92.50 - 78.90}{78.90 - 49.30} \times 100 \div 45.9(\%), \quad \rho_d = \frac{78.90 - 49.30}{26.20} \div 1.138(\text{g/cm}^3),$ $e = \frac{26.20 - 11.00}{11.00} = 1.38, \quad Sr = \frac{92.50 - 78.90}{26.20 - 11.00} \times 100 = 89.5(\%)$ <p style="text-align: right;">$3 \times 6 + 2 = 20$</p>
Question No. (3)	$(1) \sigma_1 = 600(\text{kPa}), \sigma_3 = 200(\text{kPa}),$ $(2) 2\alpha = +60^\circ \quad \alpha = +30^\circ,$ $(3) \sigma_A = 400 + 100\sqrt{3}(\text{kPa}),$ $\tau_A = 100(\text{kPa})$ <p style="text-align: right;">$4 \times 5 = 20$</p>
Question No. (4)	$(1) \sigma_{zA} = 20.0 \times 3 = 60(\text{kN/m}^2), \quad u_A = -29.4(\text{kN/m}^2), \quad \sigma'_{zA} = 60 - (-29.4) = 89.4(\text{kN/m}^2),$ $(2) \sigma_{zB} = 9.81 \times 3 + 20.0 \times 3 = 89.4(\text{kN/m}^2), \quad u_B = 9.81 \times 6 = 58.9(\text{kN/m}^2),$ $\sigma_{zB} = 89.4 - 58.9 = 30.5(\text{kN/m}^2),$ <p style="text-align: right;">$10 \times 2 = 20$</p>
Question No. (5)	$h_a = 0 + 10(\text{m}) = 10(\text{m}), h_b = 5(\text{m}),$ $Q = 0.05\left(\frac{\text{m}}{\text{s}}\right) \times 10(\text{m}^2) \times \frac{10-5}{5} = 0.5(\text{m}^3/\text{s})$ <p style="text-align: right;">$3 \times 6 + 2 = 20$</p>