

Mid Fall Term Exam for IPSE of CSE, 2014				19th, Nov., Wednesday			From: 18:15, To:19:45		
Subject		Instructor	Department, Year		Answer	Separate	Reference	Reference tools are not allowed without admission.	<div>1. Nothing</div> <div>2. Free</div> <div>3. Partly allowed</div> <div>• Textbook • Reference book</div> <div>• Calculator</div> <div>• Dictionary</div> <div>• Others [</div>
Soil Mechanics		H. Akagi	Civil & Env.	2					
Student ID			Name			Mark			

Answer all questions (1) ~ (5) on the separate answer sheet. The density of water is  $\rho_w=1.00(\text{g/cm}^3)$  and the water unit weight is  $\gamma_w=9.81(\text{kN/m}^3)$ .

(1) A 1.50 (kg) of dry soil is poured into a Eureka can and displaces 600 ( $\text{cm}^3$ ) of water (see Fig. 1). Find the density of soil particles  $\rho_s$  ( $\text{g/cm}^3$ ).

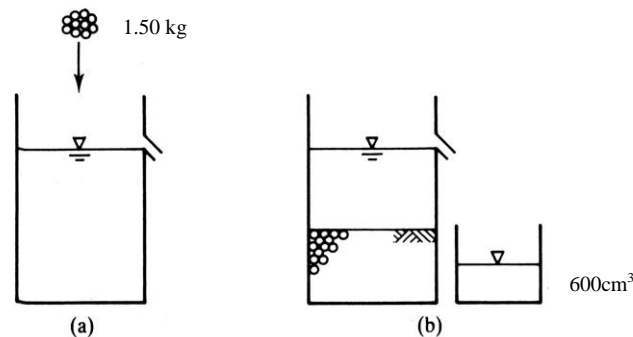


Fig.1

A 160 (g) of the same dry soil is poured uniformly into an empty impermeable metal container and occupied 180 ( $\text{cm}^3$ ) (see Fig.2). The area of the inside cross section of the container is  $A=30.0$  ( $\text{cm}^2$ ). Find the void ratio  $e$  of the dry soil in the container.

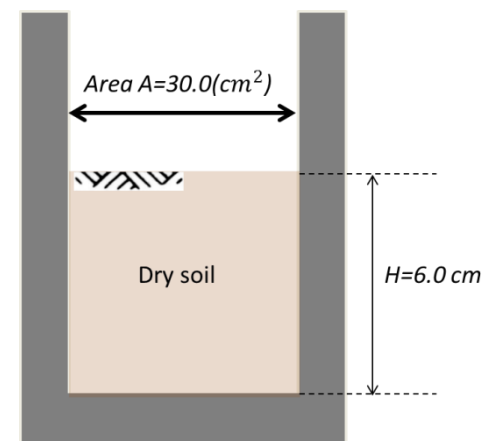


Fig.2

(2) Table 1 shows the results of the density test and water contents test of soil.

Find the density of soil particle  $\rho_s(\text{g/cm}^3)$ , wet density of soil  $\rho_t(\text{g/cm}^3)$ , water content  $w(\%)$ , dry density of soil  $\rho_d(\text{g/cm}^3)$ , void ratio  $e$  and saturation ratio  $S_r(\%)$ .

Table 1

Types of test	Results	
Density of grains	Volume of the soil particle	11.00 $\text{cm}^3$
Water content	Mass of the wet specimen and container	92.50 g
Wet density	Mass of the specimen and container after drying at 105°C	78.90 g
	Mass of the container	49.30 g
	Volume of the wet specimen	26.20 $\text{cm}^3$

(3) Fig. 2 shows the state of stresses in an element of soil. (Positive shear stress and angles indicate counterclockwise direction.)

1) Calculate the maximum and minimum principal stresses  $\sigma_1(\text{kPa})$  and  $\sigma_3(\text{kPa})$ .

2) Calculate the angle  $\alpha(\text{degrees})$  formed by the plane of the maximum principal stress and the horizontal plane.

3) The angle between plane A and horizontal plane within an element of soil is +45 degrees as shown in Fig. 2. Calculate normal and shear stresses  $\sigma_A(\text{kPa})$  and  $\tau_A(\text{kPa})$  acting on the plane A.

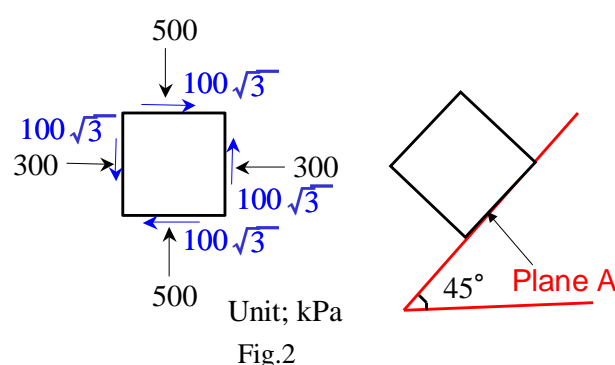


Fig.2

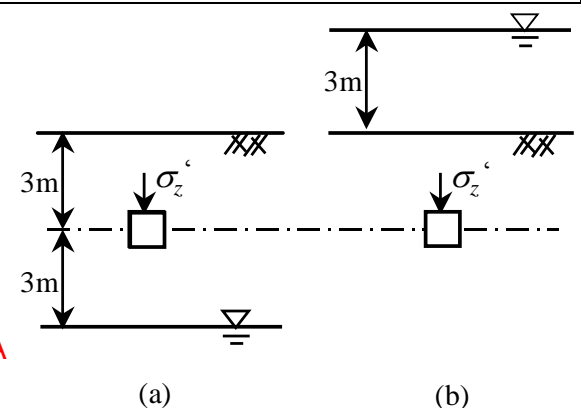


Fig.3

(4) The clay deposit in Fig.3 has unit weight  $\gamma_{\text{sat}}=20(\text{kN/m}^3)$  and the soil remains saturated if the pore pressures become negative.

1) When water table is 6 m below ground level as shown in Fig.3(a), find the normal effective stress  $\sigma'_{zA}(\text{kN/m}^2)$  at a depth of 3m.

2) When water table is 3 m above ground level as shown in Fig.3(b), find the normal effective stress  $\sigma'_{zB}(\text{kN/m}^2)$  at a depth of 3m.

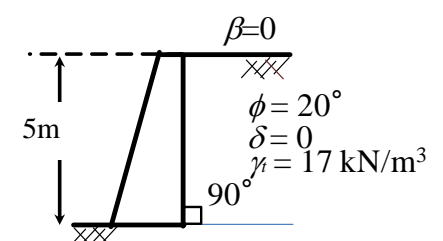


Fig.4

(5) Calculate the force due to active earth pressure  $P_A(\text{kN/m})$  and the force due to passive earth pressure  $P_P(\text{kN/m})$  acting on the retaining wall shown in Fig.4. The friction angle of soil behind the wall is  $\phi=20^\circ$  and the unit weight of soil is  $\gamma_t=17.0\text{kN/m}^3$ . The friction angle of wall surface is  $\delta=0^\circ$ .

## International Program, Department of Civil and Environmental Engineering

## Answer sheet for Mid Fall Term Exam, Soil Mechanics, 2014

Student ID \_\_\_\_\_ Name \_\_\_\_\_ Mark \_\_\_\_\_

Question No. (1)	$\rho_s = \frac{1500(\text{g})}{600(\text{cm}^3)} = 2.5(\text{g/cm}^3),$ $v_s = \frac{160(\text{g})}{2.5(\text{g/cm}^3)} = 64(\text{cm}^3), v_v = 180 - 64 = 116(\text{cm}^3), e = \frac{116}{64} \div 1.81$
	10×2=20
Question No. (2)	$\rho_s = \frac{78.90 - 49.30}{11.00} \div 2.69(\text{g/cm}^3), \quad \rho_t = \frac{92.50 - 49.30}{26.20} = 1.65(\text{g/cm}^3),$ $w = \frac{92.50 - 78.90}{78.90 - 49.30} \times 100 \div 45.9(\%), \quad \rho_d = \frac{78.90 - 49.30}{26.20} \div 1.138(\text{g/cm}^3),$ $e = \frac{26.20 - 11.00}{11.00} = 1.38, \quad Sr = \frac{92.50 - 78.90}{26.20 - 11.00} \times 100 = 89.5(\%)$
	3×6+2=20
Question No. (3)	$(1)\sigma_1 = 600(\text{kPa}), \sigma_3 = 200(\text{kPa}),$ $(2)2\alpha = +60^\circ \quad \alpha = +30^\circ,$ $(3)\sigma_A = 400 + 100\sqrt{3}(\text{kPa}),$ $\tau_A = 100(\text{kPa})$
	4×5=20
Question No. (4)	$(1)\sigma_{zA} = 20.0 \times 3 = 60(\text{kN/m}^2), \quad u_A = -29.4(\text{kN/m}^2), \sigma'_{zA} = 60 - (-29.4) = 89.4(\text{kN/m}^2),$ $(2)\sigma_{zB} = 9.81 \times 3 + 20.0 \times 3 = 89.4(\text{kN/m}^2), u_B = 9.81 \times 6 = 58.9(\text{kN/m}^2),$ $\sigma_{zB} = 89.4 - 58.9 = 30.5(\text{kN/m}^2),$
	10×2=20
Question No. (5)	$P_A = \frac{1}{2} \times \gamma_t \cdot H^2 \times \tan^2\left(45^\circ - \frac{\varphi}{2}\right) = \frac{1}{2} \times 17.0 \times 5^2 \times \tan^2(45^\circ - 10^\circ) = 104(\text{kN/m})$ $P_P = \frac{1}{2} \times \gamma_t \cdot H^2 \times \tan^2\left(45^\circ + \frac{\varphi}{2}\right) = \frac{1}{2} \times 17.0 \times 5^2 \times \tan^2(45^\circ + 10^\circ) = 433(\text{kN/m})$